

Design Load: This bridge is designed for KYHL-93 live load, (i.e. 1.25xAASHTO HL93 live load). This bridge is designed for a future wearing surface of 15 psf.

Design Method: All reinforced concrete members are designed to be equivalent or greater than the load and resistance factor design method as specified in the current AASHTO Specifications.

#### Materials Design Specifications:

f'c = 3500 psi For Class "A" Reinforced Concrete For Class "AA" Reinforced Concrete f'c = 4000 psi For Steel Reinforcement fy = 60000 psi fv = 50000 psi

Material Specifications: AASHTO Specifications or ASTM, current edition, as designated below shall govern the materials furnished.

AASHTO MI53 Premoided Cork Filler, Type II

AASHTO M-31 Deformed and Plain Billet-Steel for Concrete Reinforcement,

Preformed Cork Expansion Joint Material: Preformed Cork Expansion Joint Material shall conform to subsection 807.04.02 (Type II) of the Kentucky Department of Highways Standard Specifications

Payment for Precast Concrete Beams: The basis of payment for the Prestressed Concrete Beams shall be at the contract unit price per linear foot of beam, in accordance with the specifications.

Slope Protection: Slope Protection at abutments shall be dry cyclopean stone riprap in accordance with the plans and specifications. Geotextile Fabric, Class I shall be placed between the embankment and the slope protection in accordance with Standard Specifications 214 and 843, Payment for Geotextile Fabric, Class I, shall be considered incidental to the unit price bid for Dry Cyclopean Stone Riprap.

Stay-In-Place Metal Forms: Stay-In-Place Metal Forms may be used on bridge decks under the following additional conditions:

The valleys of forms shall be filled with trimmed styrofoam to eliminate increased dead load from concrete.

of the specifications and of the type shown on the pile record sheet.

The welding shall be performed by a certified welder.

Pilina: Pilina shall be driven to practical refusal as defined on the pile record sheet.

Test piles shall be driven where designated on the plans to determine the length of pile required.

All test piles shall be accurately located so that they may be used in the finished structure.

Contrary to the standard drawings for steel piling, mill test reports are not required to be

<u>Pile Points:</u> Provide pile points for all piles. Pile points shall be in accordance with Section 604

Wind Load: This bridge is designed for a wind load based on a wind velocity of 100 mph.

#### **General Notes**

 $\underline{\text{Concrete}}$ : Class "AA" Concrete is to be used throughout the superstructure and in the portions of the substructure above the tops of caps. Class "A" concrete is to be used in the substructure below the caps.

Reinforcement: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Any reinforcing bars designated by suffix "e" in the plans shall be epoxy coated in accordance with section 811.10 of the Standard Specifications. Any reinforcing bars designated by suffix 's' in a Bill of Reinforcement shall be considered a stirrup for purposes of bend diameters.

<u>Construction Identification</u>: The names of the Prime Contractor and the Sub-Contractor shall be imprinted in the concrete with linch letters at a location designated by the engineer. The contractor shall furnish all plans, equipment and labor necessary to do the work for which no direct payment will be made.

Beveled Edges: All exposed edges shall be beveled  $\frac{y}{4}$ , unless otherwise shown.

Completion of the Structure: The contractor is required to complete the structure in accordance with the plans and specifications, Material, labor, or construction operations not otherwise specified, are to be included in the bid item most appropriate to the work involved. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of existing structures, phase construction, incidental materials, labor, or anything else required to complete the structure.

Shop Drawings: The fabricator shall submit all required shop plans, by email to SHOP\_ 003B00029N@docs.e-Builder.net, for review. These submissions shall depict the shop plans in .PDF format, as either II'x17" or 22"x36" sheets. Designers will make review comments on these electronic submissions as needed and, if required, shall return them to the fabricator for corrections and resubmittal. Upon acceptable reconciliation of all comments, files shall be sent to the Bridging Kentucky Shop Plan Coordinator for distribution. Only plans submitted directly to the Shop Plan Coordinator will be distributed. Additionally, only plans electronically stamped 'Distributed by The Bridging Kentucky Program Team' are to be used for fabrication. While this process does not require the submission of paper copies, the Engineer of Record reserves the right to require such copies on a case by case basis.

When any changes to the design plans are proposed, the shop drawings reflecting these changes shall be submitted through the process above.

Note: The designation in the email 003B00029N refers to the Bridge ID number which is located on the Title Sheet, RI of the Bridge Plans. Example: SHOP\_003B00029N@docs.e-Builder.net

Utilities: The contractor shall be responsible for locating any and all existing utilities prior to excavation of material or installation of guardrail or other construction activities that may involve utilities (overhead or underground).

<u>Verifying Field Conditions:</u> The contractor shall field verify all dimensions before ordering material. New material that is unsuitable because of variations in the existing structure shall be replaced at the contractor's expense.

Dimensions: Dimensions are for a normal temperature of 60 degrees fahrenheit. Layout dimensions are harizantal dimensions.

Slab Pour Sequences: Ensure the entire superstructure slab is poured continuously, out to out, before allowing any concrete to set.

#### Concrete Sealer:

Apply concrete sealer in accordance with the Special Note for Concrete Sealing and to the limits as indicated in the plans.

Elastomeric Bearing Pads: Elastomeric Bearing Pads shall conform to the AASHTO Standard Specifications for Highway Bridges.

Bearings shall be Low Temperature Grade 3 with a shear modulus between 95 psi and 130 psi and shall be subjected to the load testing requirements corresponding to Design Method B. The cost of bearing pads is to be included in the unit price per linear feet for Precast

Temporary Supports: Temporary Supports or shoring will not be permitted under the beams when pouring the concrete deck slab or when taking "top of beam" elevations.

Armored Edge: Fabricate armored edge to match cross slope and parabolic crown at each end of bridge.

Foundation Preparation: Foundation Preparation shall be in accordance with Section 603

Foundation excavations should be properly braced/shored to provide adequate safety to persons working in or around excavations. Bracing should be performed in accordance with applicable federal, state and local guidelines.

Temporary shoring, sheeting cofferdams, and/or dewatering methods may be required to facilitate foundation construction. It should be anticipated that groundwater will be encountered at foundation locations within the flood plain.

Temporary shoring, bracing, sheeting, cofferdams and dewatering shall be included in the

<u>Structural Granular Backfill:</u> Materials for Structural Granular Backfill shall be in accordance with Section 805 of the Specifications.

Contrary to the Specifications, Structural Granular Backfill will not be measured for payment but shall be included in the Lump Sum Bid for Foundation Preparation

Spread Footing: Based on a review of the existing subsurface conditions and anticipated structural loads, it is recommended that rock bearing foundation system consisting of spread footings be used for pier substructure elements. A presumptive begring resistance of 20 ksf on unweathered bedrock is being recommended.

Excavation for footings at the structure locations should be level and free of loose, water softened material, etc. Additional rock excavation to achieve suitable bearing conditions may be required depending upon topography and bedrock weathering conditions.

Solid rock excavation will be required for installation of the substructure's spread footings. The contractor shall take care during blasting and other excavation methods to avoid over-breakage and damage to the bedrock beneath the footings.

Footing excavations in bedrock shall be cut neatly so that no forming or backfilling is necessary in the construction of the portions of the footings located in rock. Concrete shall be placed directly against the cut rock faces. Mass concrete should be placed in the excavation from the top of the footing to the bedrock surface where the footing does not extend to the bedrock surface.

Bearing elevation of footings may be adjusted at the discretion of the Engineer if competent, unweathered bedrock is found at a higher elevation than specified for the respective substructure element. The top of new spread footings should be fully embedded into unweathered bedrock. At a minimum, two-foot embedment into competent bedrock shall be maintained.

Prior to placement of any concrete or reinforcing steel in a foundation excavation, the excavation bottom should be clean and all soft, wet, or loose materials should be removed. In no case should concrete be placed upon compressible or water-softened materials. Any clay seams or suspect weak materials at or near the bearing elevation will need to be undercut and replaced with mass concrete.

Concrete placement for footings should be placed as soon as practical after completion of the footing excavation. If the bedrock becomes softened at bearing elevation, the softened material should be undercut to unweathered material prior to placement of reinforcing steel and concrete. Seasonal groundwater fluctuations may cause groundwater infiltration into the footing excavation, and a dewatering method may be necessary.

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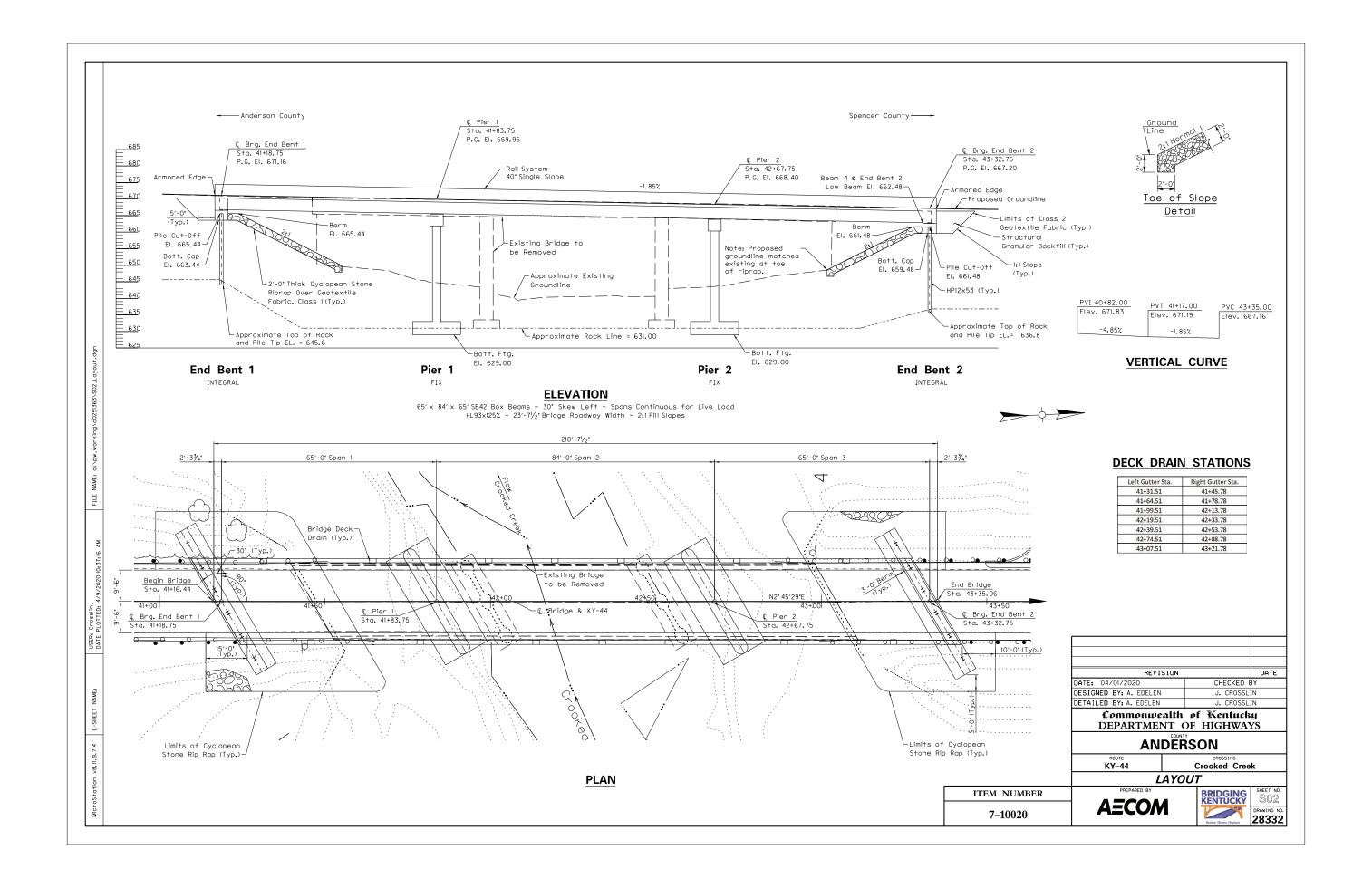
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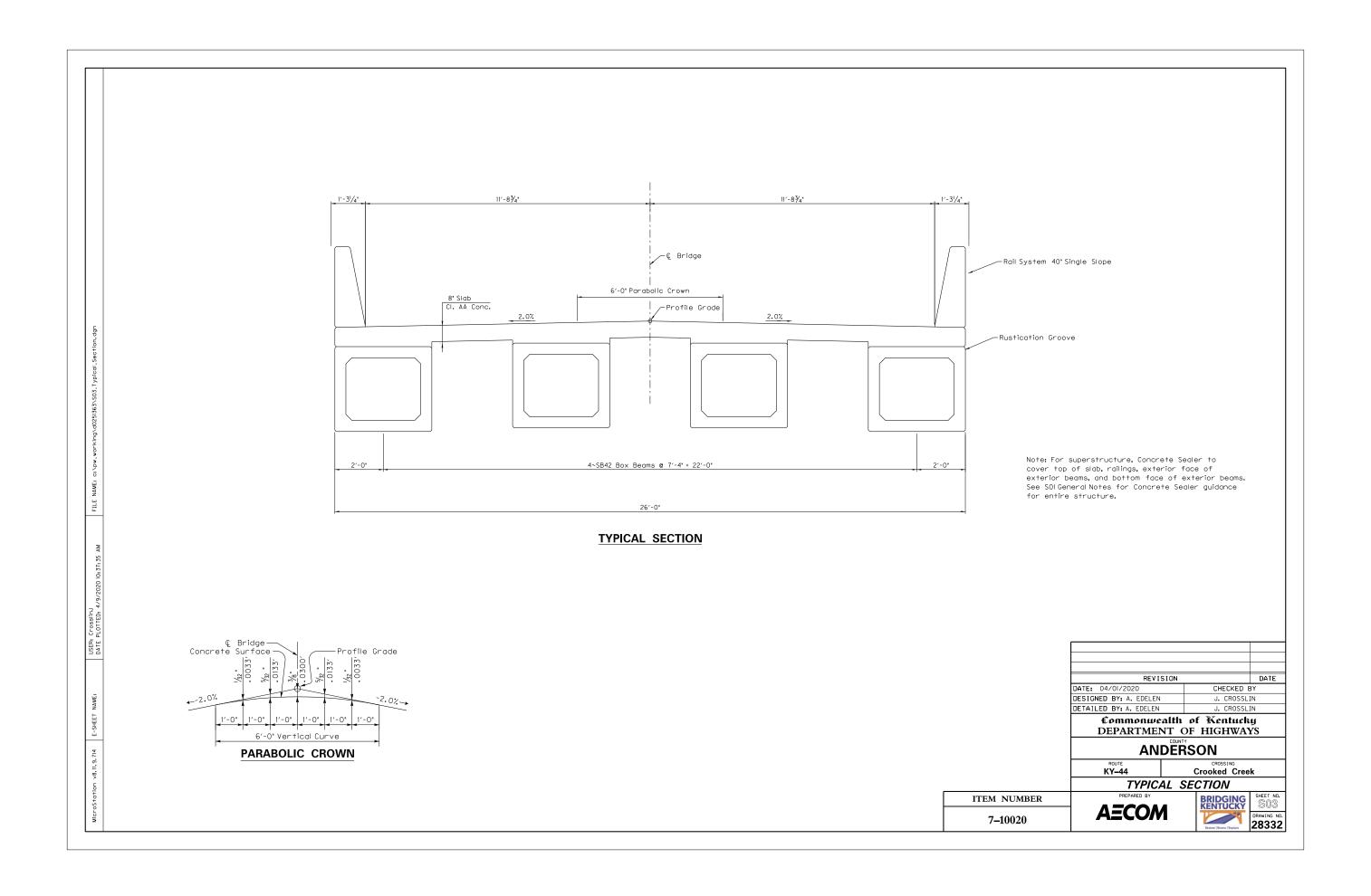
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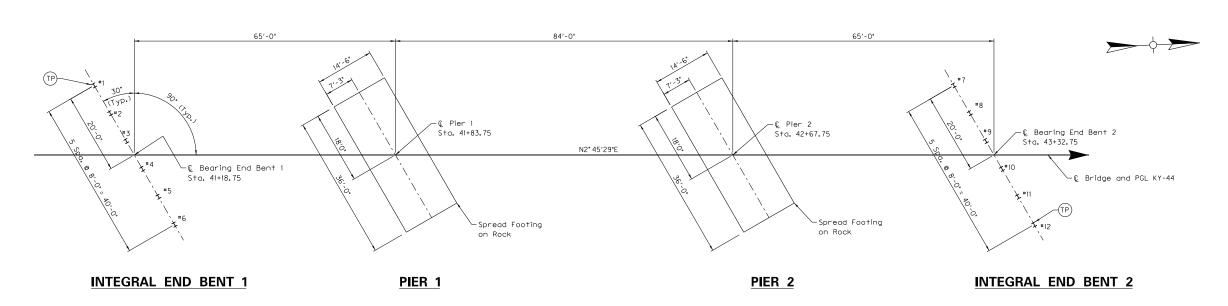




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## **FOUNDATION LAYOUT**

ı	PILE RECOF	D FOR PO	int bearin	IG PILES
Pile No.	Pile Cut–off Elevation	Pile Length In Place	Point of Pile Elevation As Driven	Design Axlal Load
	FEET	FEET	FEET	TONS
- 1	665.44			100
2	665.44			100
3	665.44			100
4	665.44			100
5	665.44			100
6	665.44			100
7	661.48			100
8	661.48			100
9	661.48			100
10	661.48			100
$\equiv$	661.48			100
12	661.48			100

#### Notes

- A diesel pile driving hammer with a rated energy between 13.5 foot-kips and 20.1 foot-kips will be required to drive 12x53 steel H-piles to practical refusal without encountering excessive blow counts or damaging the piles. The Contractor shall submit the proposed pile driving system to the Engineer for approval prior to the installation of the first pile. Approval of the pile driving system by the Engineer will be subject to satisfactory field performance of the pile driving procedures.

  2. The installation of the pile foundations should conform to current
- AASHTO LRFD Bridge Design Specifications, and Section 604 of the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction.
- 3. The Kentucky Transportation Cabinet recommends that protective pile points be used on end bearing piles to allow for embedment into the top of bedrock. Use of reinforced pile points capable of penetrating boulders and hard layers which may be encountered is recommended. Installation of pile points should be in accordance with Section 604 of the Kentucky Standard Specifications for Road and Bridge Construction,
- 4. For spread footing on rock, see general notes sheet.

## Definitions of Terms

PILE CUT-OFF ELEVATION: ELE tion of the top of pile in the finished structure. PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off Elevation in the finished structure.

POINT OF PILE ELEVATION AS DRIVEN: Actual point of pile elevation in the finished structure. structure.

DESIGN AXIAL LOAD: Load carried by each pile as estimated from structural design calculations for Factored LRFD Loadings.

CALCULATED FIELD BEARING: Contrary to Section 604.03.07 of the Standard Specifications, in place bearing values are not required for piles bearing on rock when driven to practical refusal.

### **Driving Criteria**

DRIVING CRITERIA: Drive point bearing piles to practical refusal.

DRIVING CRITERIA: Drive point bearing piles to practical refusal.

PRACTICAL REFUSAL: For this project minimum blow requirements are reached after total penetration becomes ½ or less for 10 consecutive blows, practical refusal is obtained after the pile is struck an additional 0 blows with total penetration of ½ or less. Advance the production piling to the driving resistances specified obove and to depths determined by test pilets) and subsurface data sheetis). Immediately cease driving operations if the pile visibly yields or becomes damaged during driving, if hard driving is encountered because of dense strata or an obstruction, such as a boulder before the pile is advanced to the depth anticipated, the Engineer will determine if more blows than the average driving resistance specified for practical refusal is required to further advance the pile. Drive additional production and test piles if directed by the Engineer.

### Field Data

For each pile, the Project Engineer shall record the following on this sheet: Pile Length in Place and Point of Pile Elevation as Driven.

Kentucky Transportation Cabinet Director, Division of Structural Design 3rd Floor East 200 Mero Street Frankfort, KY 40622

This pile record does not replace other pile records the Project Engineer is required to keep and submit.

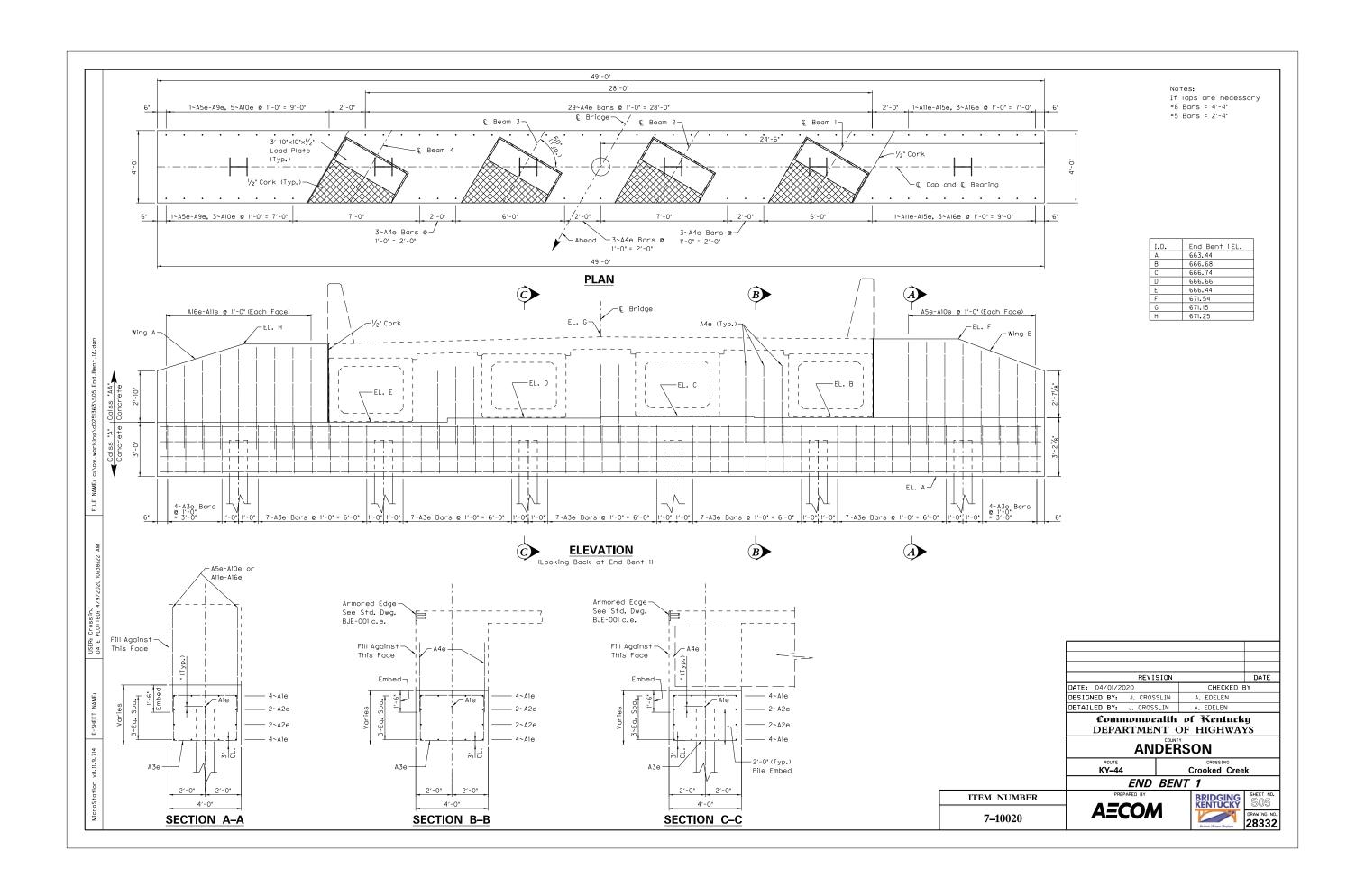
Use HP 12x53 in accordance with BPS-003, c.e.

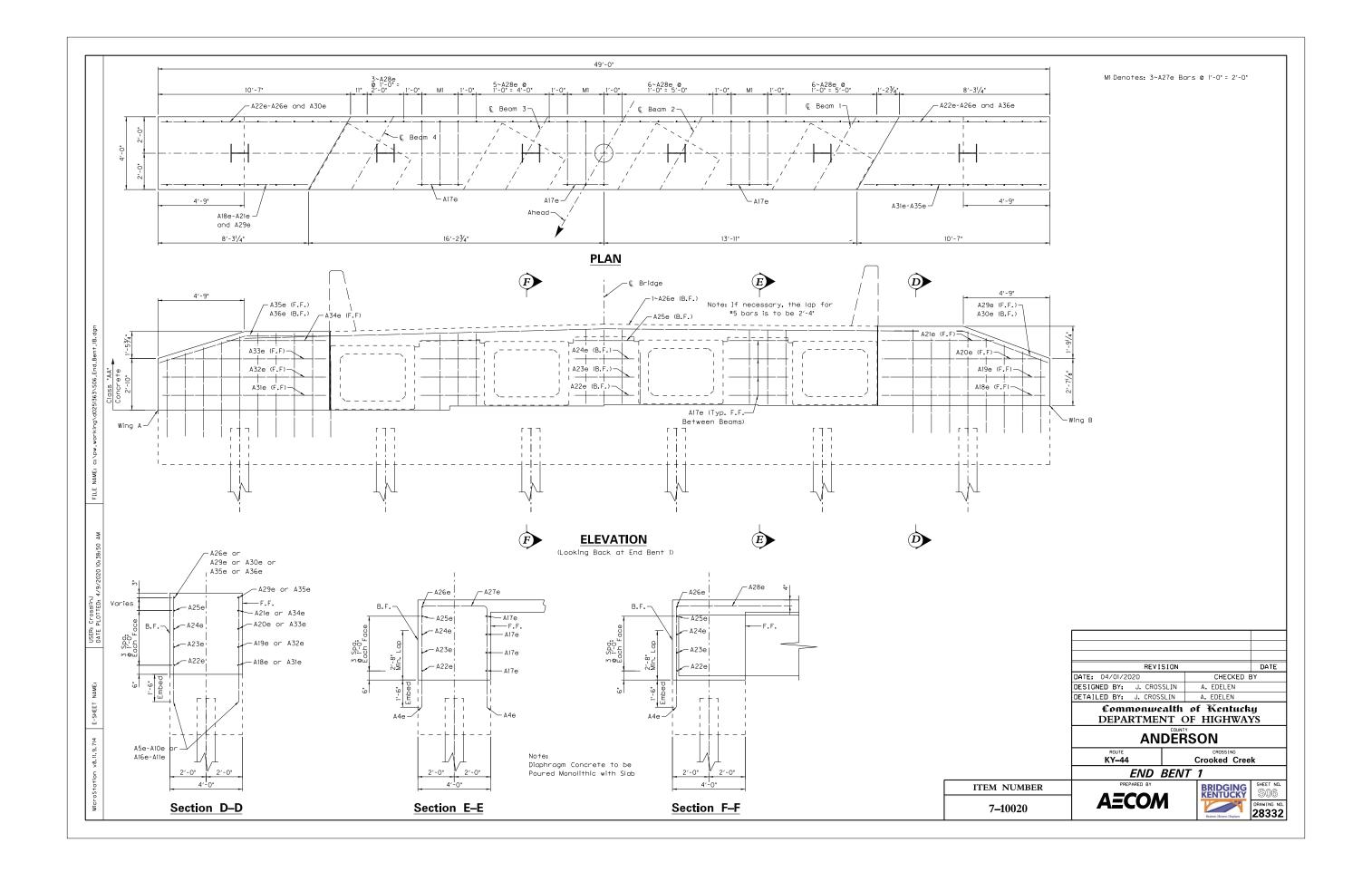
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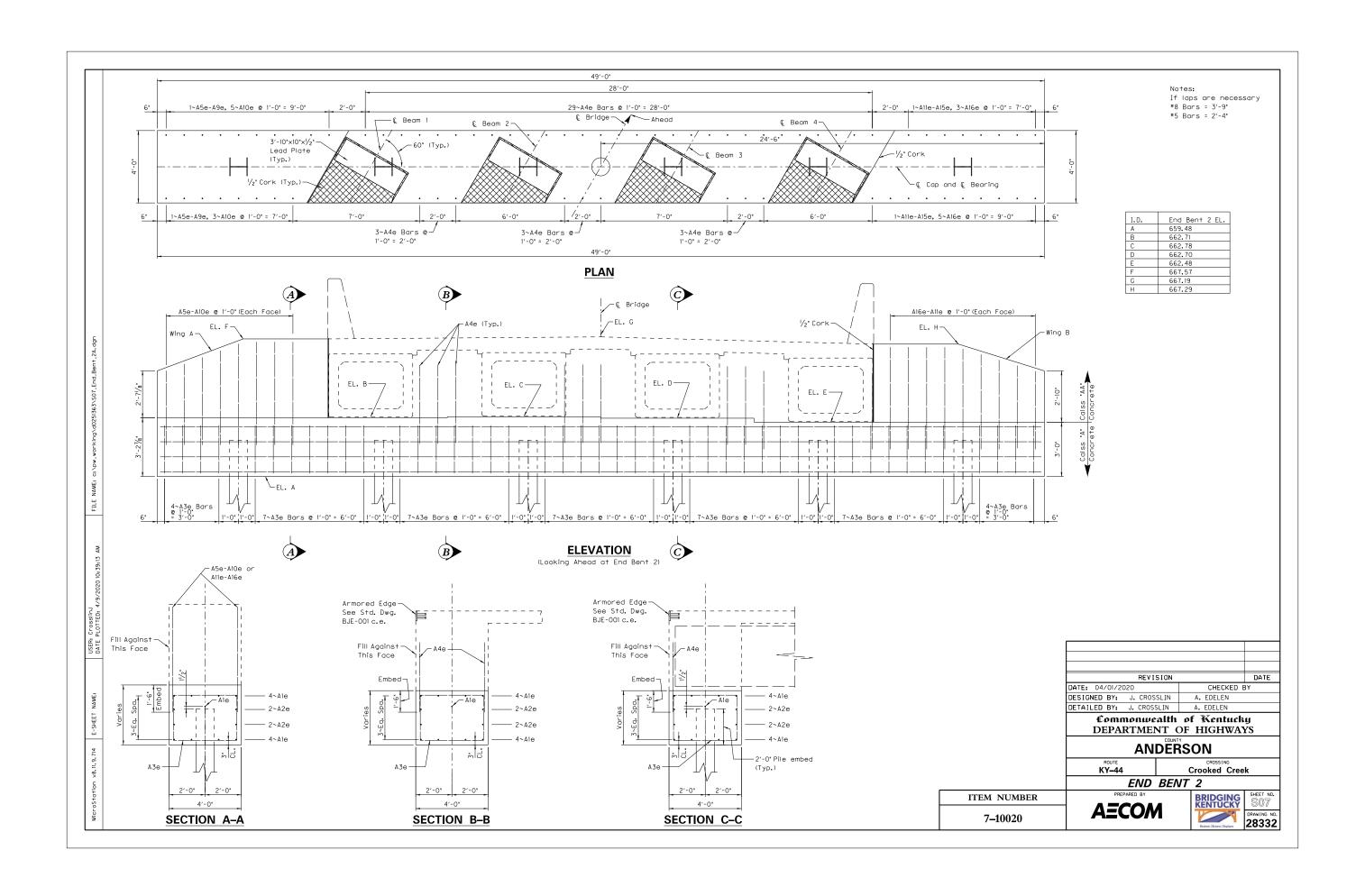
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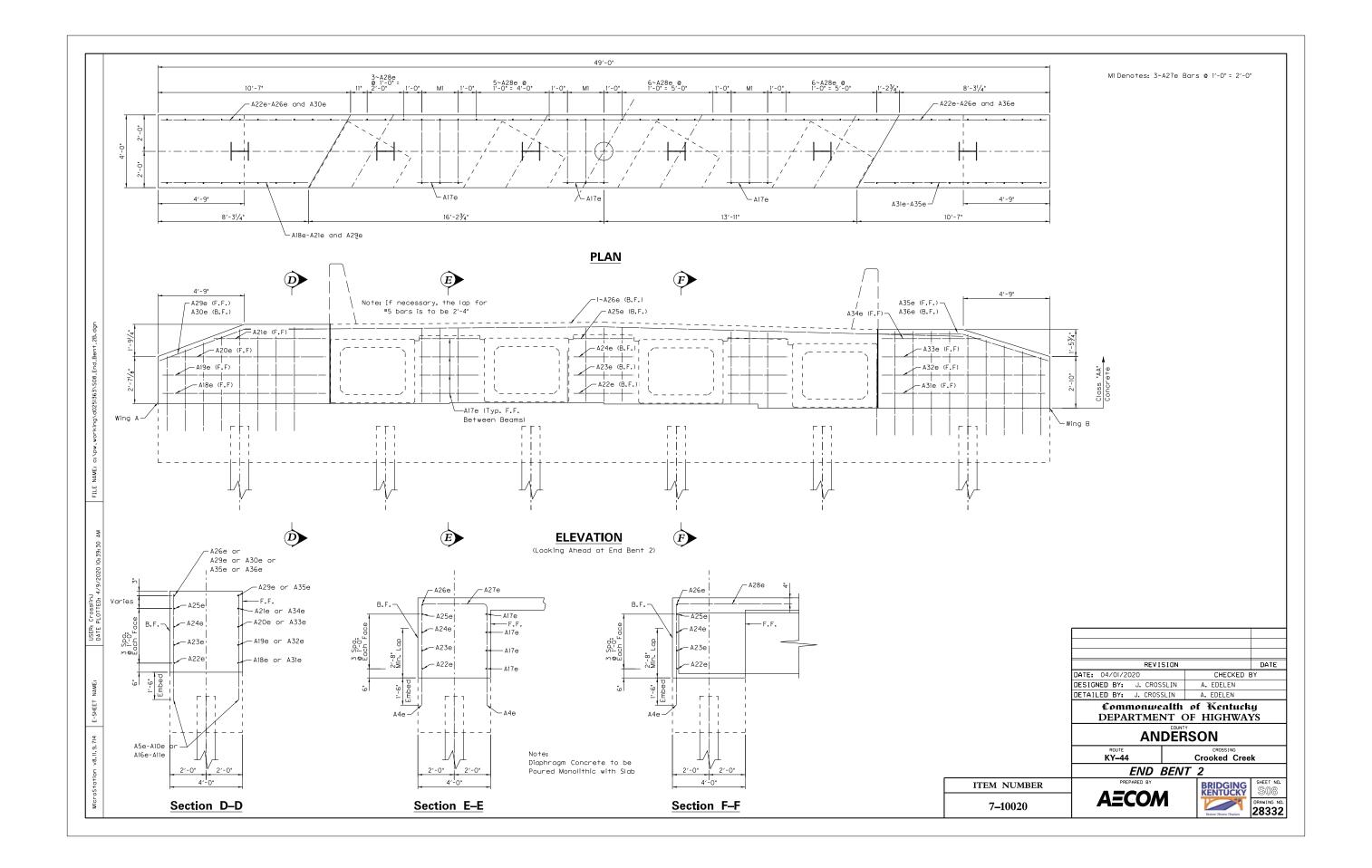
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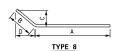




		NUMBER		Ler	ngth			a		b		С	//	d
Mark	Type	REQD.	SIZE	ft	in	LOCATION	ft	in	ft	in	ft	in	ft	in
A1e	Str	9	8	48	8	Cap								
A2e	Str	4	5	48	8	Cap Sides								
АЗе	14	43	5	13	6	Cap Stirrups	2	7	3	8				
A4e	Str	38	5	4	4	Cap Dowell								
A5e	Str	2	5	4	1	Wing A Vertical								
A6e	Str	2	5	4	5	Wing A Vertical								
A7e	Str	2	5	4	10	Wing A Vertical								
A8e	Str	2	5	5	2	Wing A Vertical								
A9e	Str	2	5	5	7	Wing A Vertical								
A10e	Str	8	5	5	8	Wing A Vertical								
A11e	Str	2	5	4	3	Wing B Vertical								
A12e	Str	2	5	4	7	Wing B Vertical								
A13e	Str	2	5	4	11	Wing B Vertical								
A14e	Str	2	5	5	3	Wing B Vertical								
A15e	Str	2	5	5	6	Wing B Vertical								
A16e	Str	8	5	5	5	Wing B Vertical								
A17e	Str	12	5	3	3.5	Diaphragm								
A18e	Str	1	5	8	0	Wing A Horizontal								
A19e	Str	1	5	8	0	Wing A Horizontal								
A20e	Str	1	5	7	8	Wing A Horizontal								
A21e	Str	1	5	5	3	Wing A Horizontal								
A22e	Str	1	5	48	8	Long Diaphragm Bars								
A23e	Str	1	5	48	8	Long Diaphragm Bars								
A24e	Str	1	5	47	11	Long Diaphragm Bars								
A25e	Str	1	5	43	2	Long Diaphragm Bars								
A26e	Str	1	5	39	2	Long Diaphragm Bars								
A27e	2	9	5	10	11	Diaphragm	3	8	3	7				
A28e	5	20	5	8	0.5	Diaphragm over Beams	4	0.5	4	0				
A29e	8	1	6	8	9	Wing A Top	5	0	3	9	1	9	3	6
A30e	8	1	6	8	9	Wing A Top	5	0	3	9	1	9	3	6
A31e	Str	1	5	10	0	Wing B Horizontal								
A32e	Str	1	5	10	0	Wing B Horizontal								
A33e	Str	1	5	9	7	Wing B Horizontal								
A34e	Str	1	5	6	5	Wing B Horizontal								
A35e	8	1	6	8	9	Wing B Top	5	0	3	9	1	9	3	6
A36e	8	1	6	8	9	Wing B Top	5	0	3	9	1	9	3	6









**BAR TYPES** 

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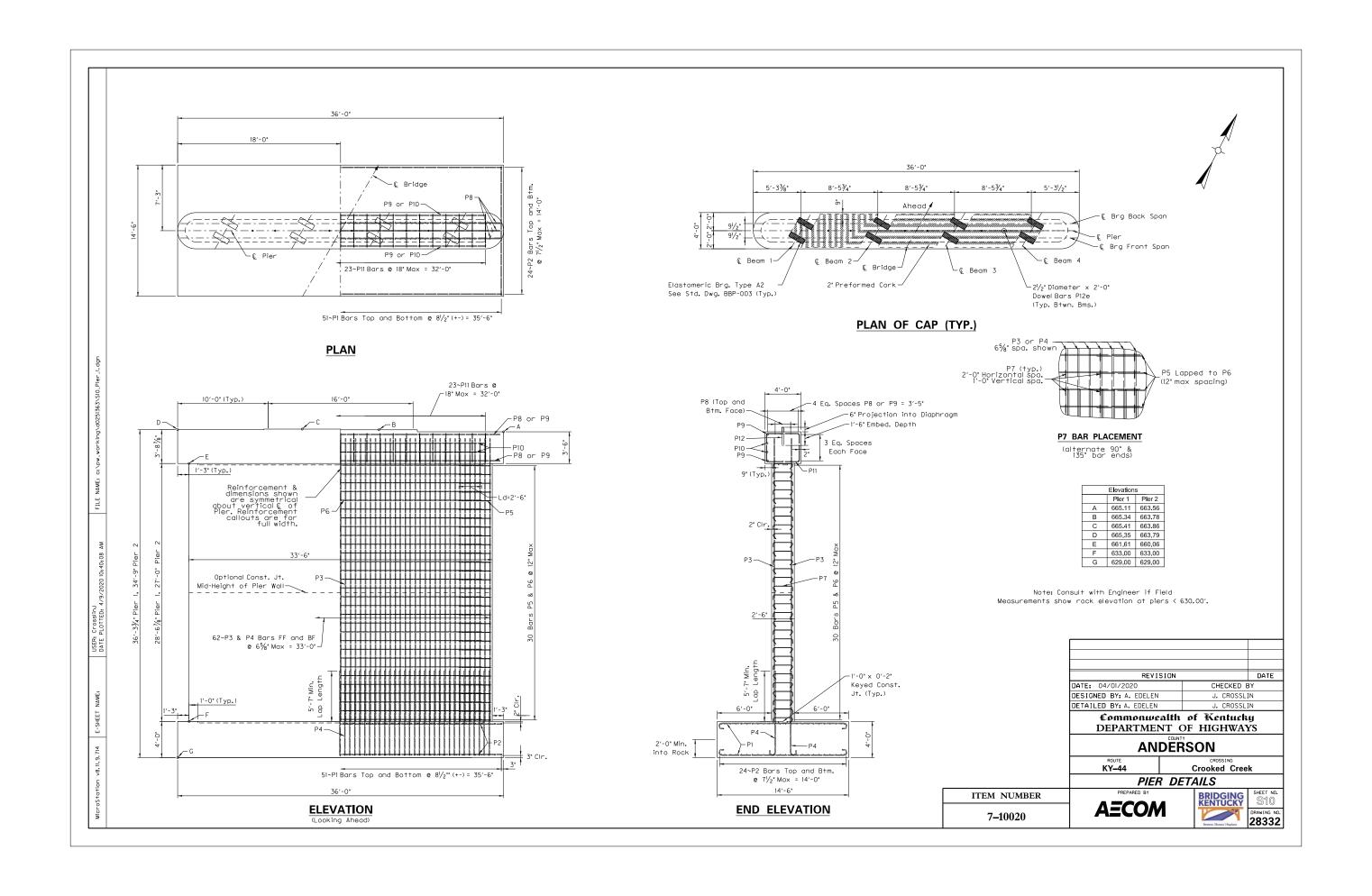
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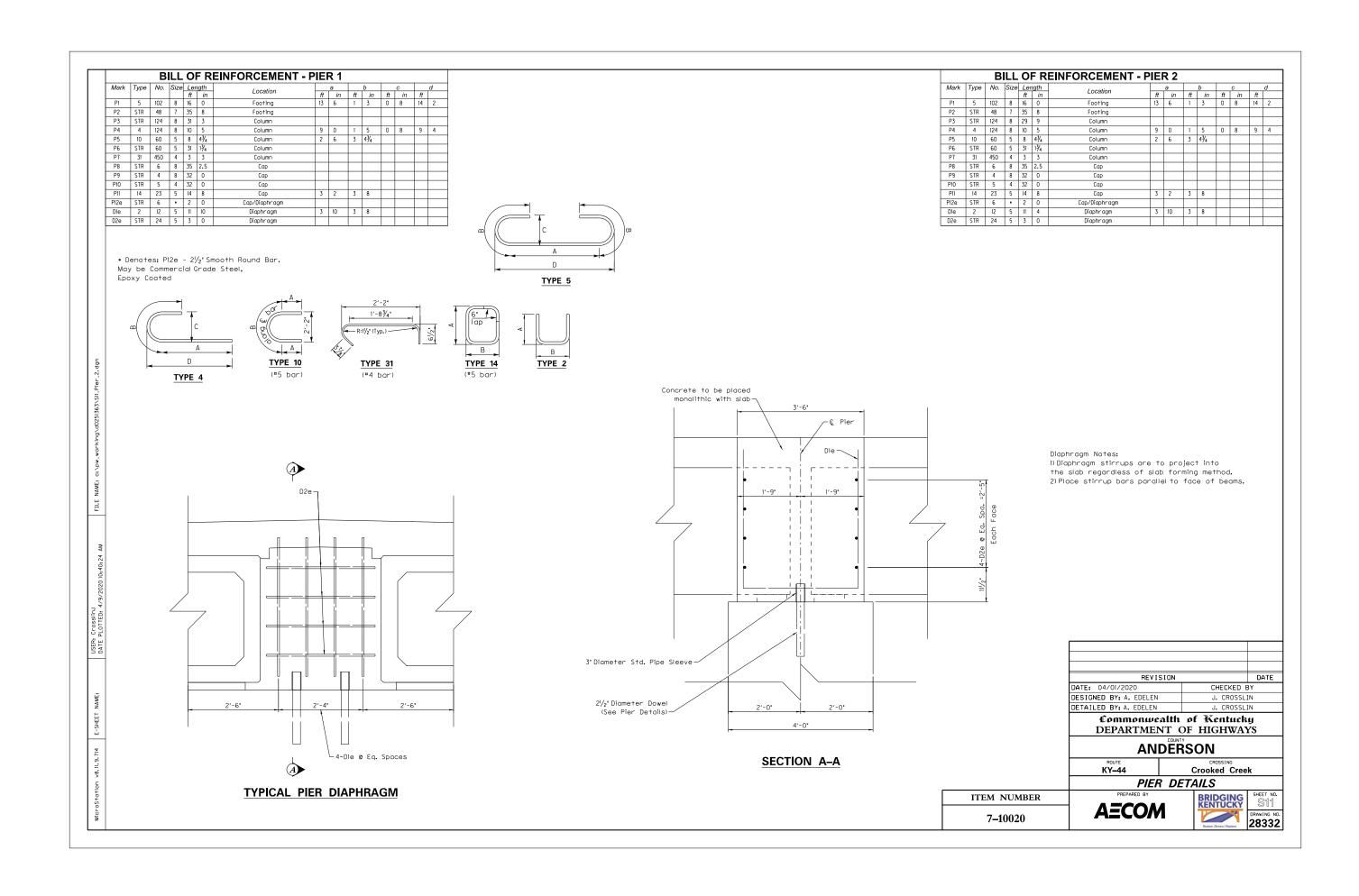
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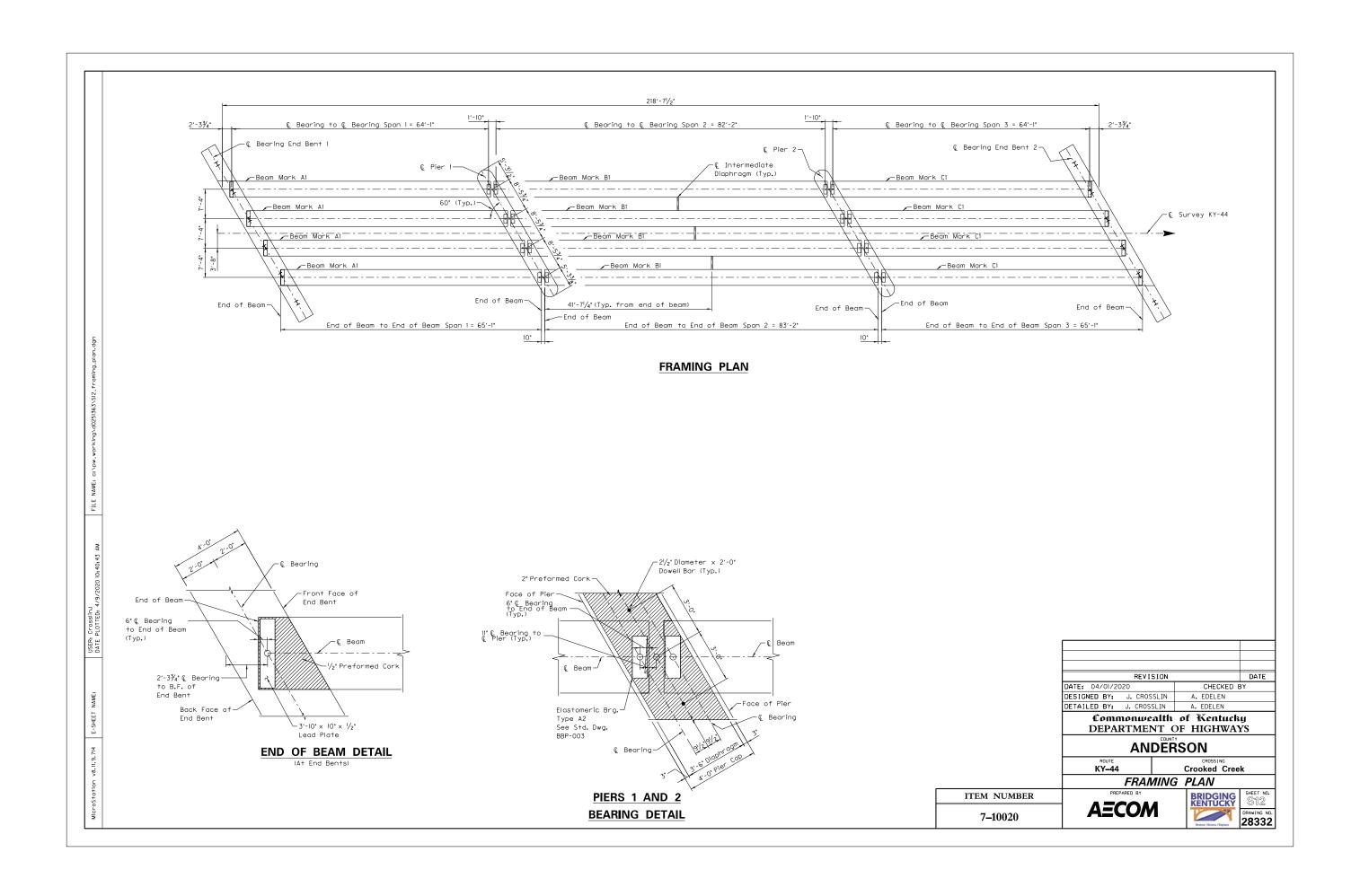
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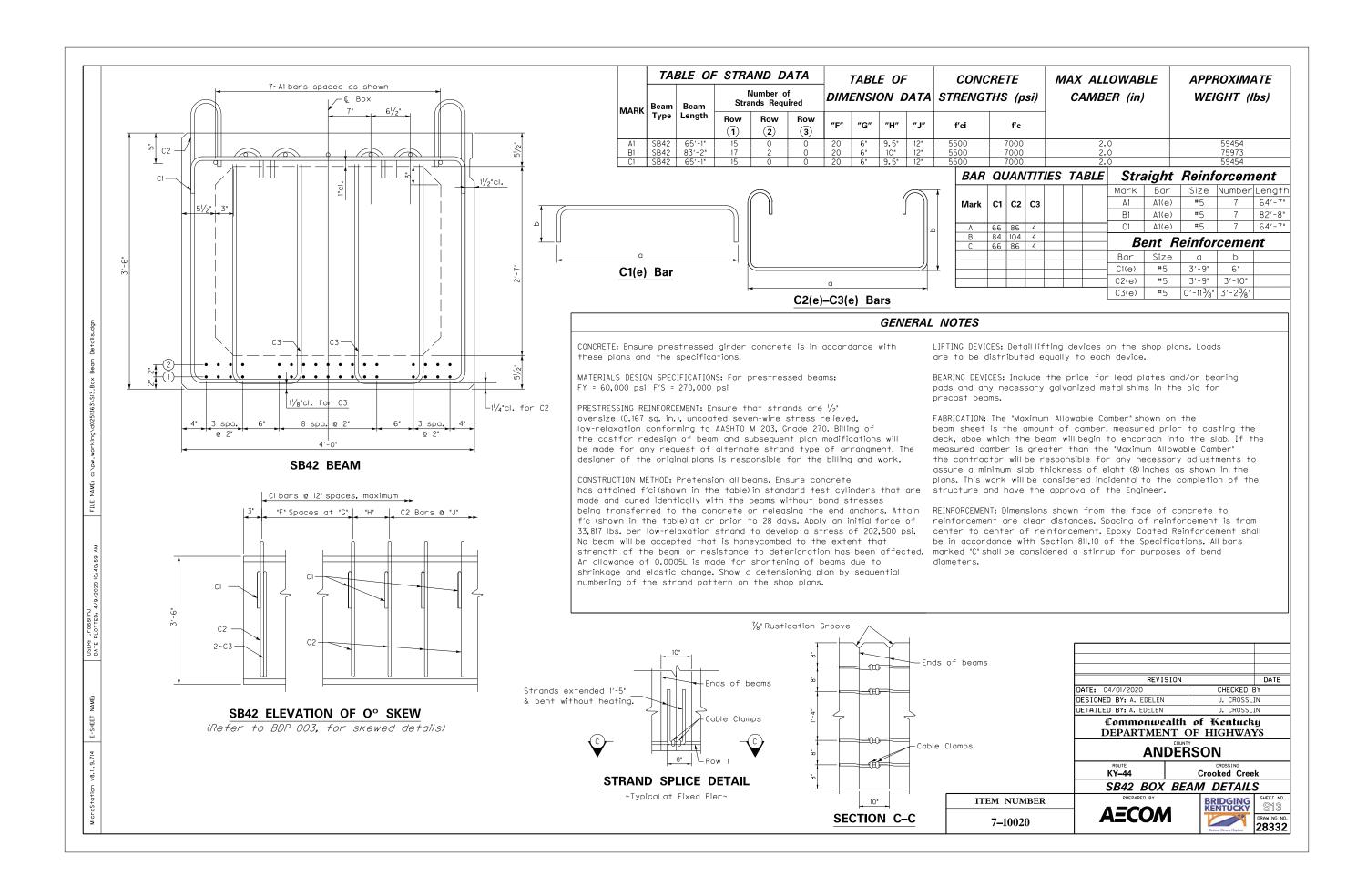
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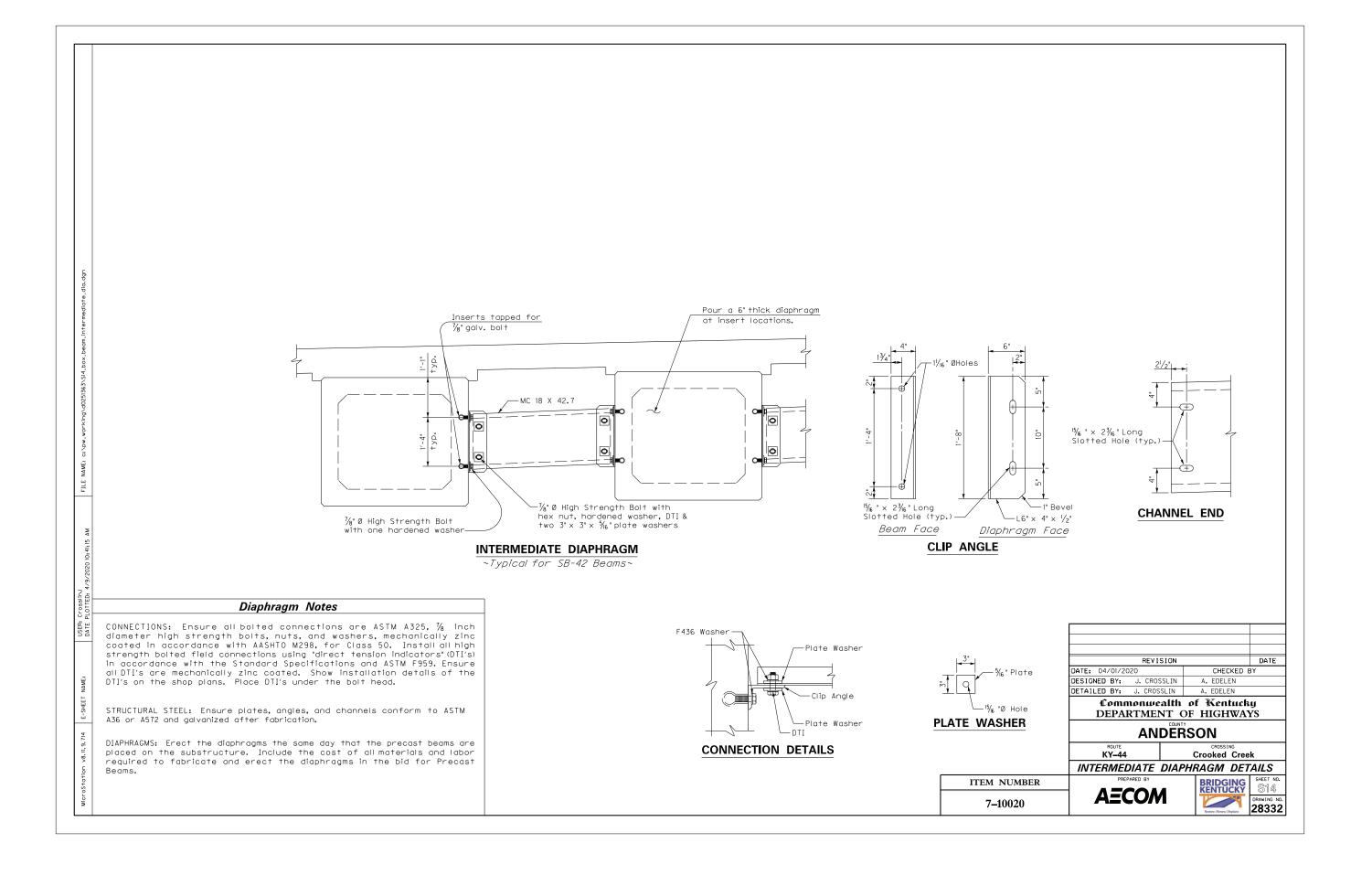


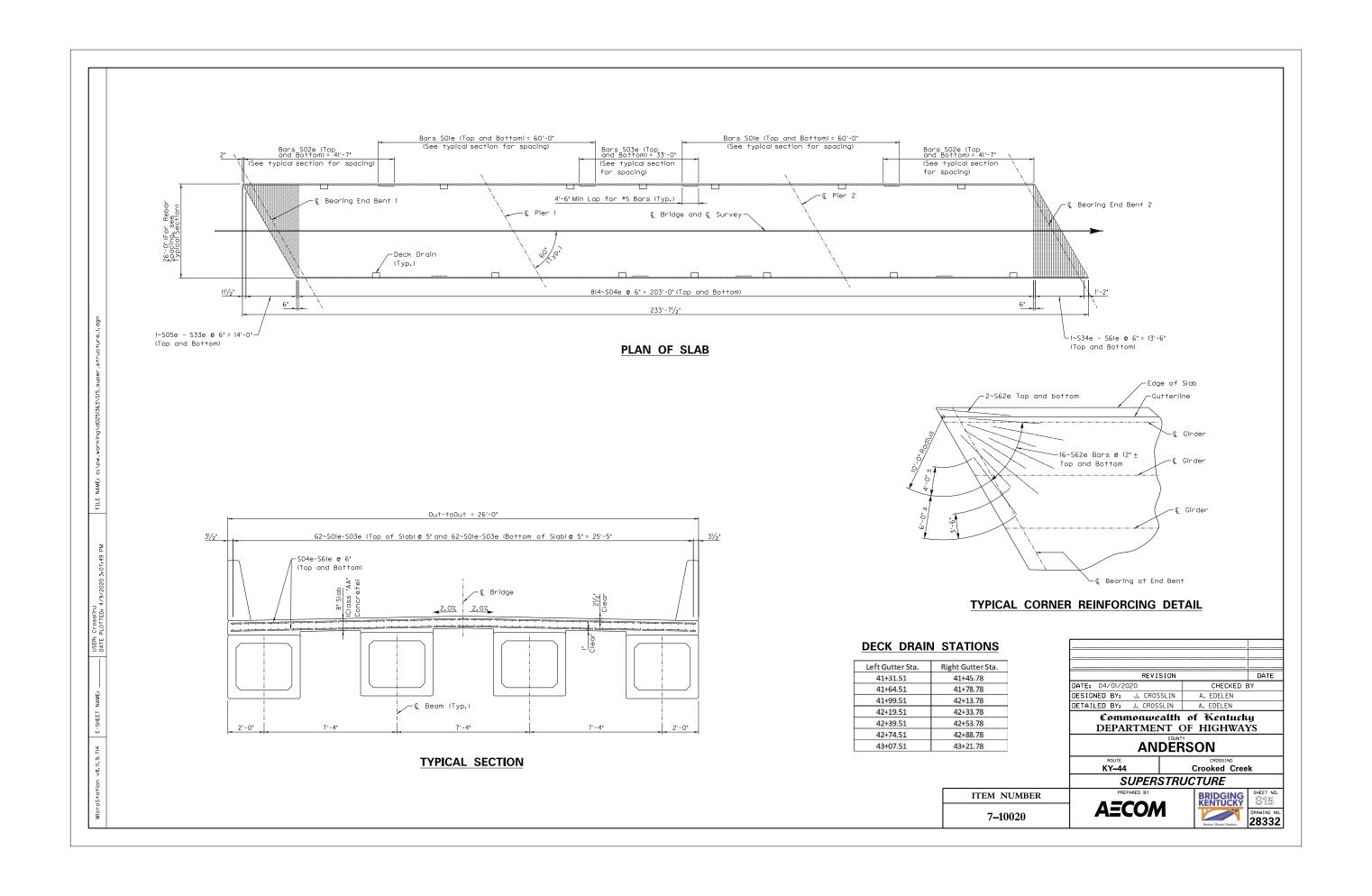






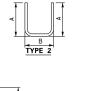




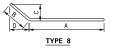


# BILL OF REINFORCEMENT FOR SLAB

			NUMBER		Len	ngth		- 1	a		b		С		d
	Mark	Type	REQD.	SIZE	ft	in	LOCATION	ft	in	ft	in	ft	in	ft	in
	S01e	Str.	248	5	60	0	Slab Top and Bot. Long.								
	S02e	Str.	248	5	41	7	Slab Top and Bot. Long.								
	S03e	Str.	124	5	33	0	Slab Top and Bot. Long.								
	S04e	Str.	814	5	25	8	Slab Top and Bot. Trans								
	S05e	Str.	2	5	1	1.875	Slab Top and Bot. Trans								
	S06e	Str.	2	5	2	0.25	Slab Top and Bot. Trans								
	S07e	Str.	2	5	2	10.625	Slab Top and Bot. Trans								
	S08e	Str.	2	5	3	9	Slab Top and Bot. Trans								
	S09e	Str.	2	5	4	7.375	Slab Top and Bot. Trans								
	S10e	Str.	2	5	5	5.75	Slab Top and Bot. Trans								
	S11e	Str.	2	5	6	4.125	Slab Top and Bot. Trans								
	S11e	Str.	2	5	7	2.5		-			<del>                                     </del>				
							Slab Top and Bot. Trans								
	S13e	Str.	2	5	8	0.875	Slab Top and Bot. Trans								
	S14e	Str.	2	5	8	11.25	Slab Top and Bot. Trans				—				
_	S15e	Str.	2	5	9	9.75	Slab Top and Bot. Trans	-							
₽.	S16e	Str.	2	5	10	8.125	Slab Top and Bot. Trans				<b>├</b>				
is	S17e	Str.	2	5	11	6.5	Slab Top and Bot. Trans				——				
P,	S18e	Str.	2	5	12	4.875	Slab Top and Bot. Trans								
2-b	S19e	Str.	2	5	13	3.25	Slab Top and Bot. Trans				——				
ē.	S20e	Str.	2	5	14	1.625	Slab Top and Bot. Trans				<u> </u>				
C+C	S21e	Str.	2	5	15	0	Slab Top and Bot. Trans								
1 ₽	S22e	Str.	2	5	15	10.375	Slab Top and Bot. Trans								
ν. v	S23e	Str.	2	5	16	8.75	Slab Top and Bot. Trans								
9	S24e	Str.	2	5	17	7.25	Slab Top and Bot. Trans								
8-9	S25e	Str.	2	5	18	5.625	Slab Top and Bot. Trans								
c:\pw_working\d0251363\S16.super_structure_2_bar_list.dgn	S26e	Str.	2	5	19	4	Slab Top and Bot. Trans								
136.	S27e	Str.	2	5	20	2.375	Slab Top and Bot. Trans								
1025	S28e	Str.	2	5	21	0.75	Slab Top and Bot. Trans								
	S29e	Str.	2	5	21	11.125	Slab Top and Bot. Trans								
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	S42e	Str.	2	5	17	11.875	Slab Top and Bot. Trans								
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%	S44e	Str.	2	5	16	3	Slab Top and Bot. Trans				<del></del>				
5 %	S45e	Str.	2	5	15	4.625	Slab Top and Bot. Trans								
I ES EL	S46e	Str.	2	5	14	6.25	Slab Top and Bot. Trans				<del></del>				
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[農田]	S48e	Str.	2	5	12	9.5	Slab Top and Bot. Trans								
DAT	S49e	Str.	2	5	11	11.125	Slab Top and Bot. Trans								
$\vdash$	S50e	Str.	2	5	11	0.75	Slab Top and Bot. Trans								
	S51e	Str.	2	5	10	2.375	Slab Top and Bot. Trans								
	S52e	Str.	2	5	9	4	Slab Top and Bot. Trans								
ایا	S53e	Str.	2	5	8	5.5	Slab Top and Bot. Trans								
AM	S54e	Str.	2	5	7	7.125	Slab Top and Bot. Trans								
ΤĒΙ	S55e	Str.	2	5	6	8.75	Slab Top and Bot. Trans								
E-SHEET NAME:	S56e	Str.	2	5	5	10.375	Slab Top and Bot. Trans								
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	S58e	Str.	2	5	4	1.625	Slab Top and Bot. Trans				$\vdash$				
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oStation v8.11.9.714	S60e	Str.	2	5	2	4.875	Slab Top and Bot. Trans								
8,	S61e	Str.	2	5	1	6.5	Slab Top and Bot. Trans	+			-				
	2016	JU.		ر	1 1	0.5	Stab Top and Dot. Italis	1	1						
5	S62e	Str	36	6	10	0	Slab Corner								1









# BAR TYPES

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DETAILED BY: J. CROSSLIN	A. EDELEN	
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DEPARTMENT OF HIGHWAYS

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ROUTE CROSSING
KY-44 Crooked Creek

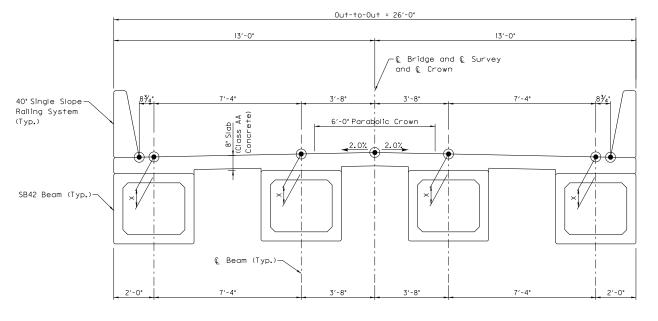
SUPER STRUCTURE BAR LIST

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TYPICAL SECTION

• Denotes: Constructon Elevations are Given at These Points

## **NOTES**

Take elevations on top of girder at points indicated by the grid layout. The beam elevations are to be read to three decimals, and entered in the tables under "Top of Girder" elevations.

Compute dimension "X" as follows: "Construction Elevation" minus "Top of Girder" elevation equals dimension "X". Construction Elevations include camber due to weight of concrete slab, barrier, and future wearing surfaces. Measuring of dimension 'X' gives the final check on beam tolerances for camber, girder damage, and errors in erection that produce reverse cambers, sags, and unsightly fascia girders.

The minimum allowable dimension "X" on a beam results in the design deck thickness (8") at the edge of the beam flange. This is calculated as the deck thickness +(half the top flange width \* the cross slope of the bridge). This is  $8"+(24\frac{1}{2}"*0.02) = 8.49" = 0.708'$ . Any necessary modifications to some or all of the "X"-dimensions must meet approval of the Engineer.

For setting templates, measure dimension "X" above top of girders for top of template. Do not set template by elevations.

Temporary supports or shoring will not be permitted under the girders when pouring the concrete floor slab or when taking 'Top of Girder'

Construct barrier curb to roadway grade. Do not add camber to barrier grade.

Note: The "Maximum Allowable Camber" shown on Sheet SII is the amount of camber, measured prior to the casting of the deck, above which the beam will begin to encroach into the slab. If the measured camber is greater than the "Maximum Allowable Camber" the contractor will be responsible for any necessary adjustments to assure a minimum slab thickness of eight (8) inches as shown in the plans. This work will be considered incidental to the completion of the structure and have the approval of the Engineer.

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DETAILED BY: J	. CROSSLIN		Α.	EDELEN		
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